

The comparison of the performance of single-equation models of turbulence kinetic energy and two k-e equation models in hydraulic simulation of dam failure on the mobile bed

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Abstract

The selection of the type of turbulence models has an important impact on the performance of the numerical models for flow simulation in the computational fluid dynamics softwares. This research is conducted to determine the effectiveness of single-equation model of turbulence kinetic energy and two-equation model of K-Epsilon in simulation of positive wave movement caused by dam failure on a mobile bed. The research method included the numerical calculations using Flow-3D software. The reserach results showed that K-Epsilon model is more accurate in predicting the pattern of the flow's free surface. Both models have the same accuracy in calculating the profile of the non-dimensional depth, velocity and acceleration of the advancing front of positive wave. Neither model could predict well the profile of mobile sediment bed. The final conclusion is that K-Epsilon turbulence model has higher efficiency in simulation of flow in dam failure on the mobile bed.

Keywords: turbulence models, turbulence kinetic energy, K-Epsilon, Flow-3D, dam failure

Introduction

Dam failure phenomenon is simply the creation of a fast unsteady flow and the flow appears as the propagation of positive and negative waves respectively to the downstream and upstream of its formation point (Chow, 1959). The study of this phenomenon is important in the sudden flood forecasting that threatens residential areas and farm land located downstream of the dam and can cause huge life and financial losses (Kocaman, 2015). The first efforts for solving the problem of dam failure were done by Ritter (1892), Dressler (1952), Whitham (1955) and Stoker (1957). These researchers presented methods, mainly based on the analytical solutions of Saint -Venant shallow water equations.

In general, various researchers used three different approaches, based on experimental modeling, computational methods and a combination of these two methods in solving the problem of dam failure. Experimental approach is mainly carried out in two general forms of dam failures on the dry bed and wet bed. The studies which are conducted by the experimental approach include Miller and Chaudhry (1989), Spinewine and Zech (2003), Cristo and Leopardi (2010), as well as Wood and Wang (2015).

Townson and Salihi (1989) used characteristics method for solving Saint – Venant equations to model positive wave movement of the dam.

Ancey et al. (2008) studied the dam failure flow on steep slopes by proposing an exact solution for shallow water equations. Ozmen-cagatay and Kocaman (2011) conducted an experimental - numerical research to study the effects of the slope change of the bed and sudden change in channel topography on the dam break flow through shallow water models and averaged Reynolds equations. Cozzolino et al. (2015) used an experimental - numerical approach for modeling the collision of shock waves caused by the dam failure for the vertical wall located in the end of downstream of the experimental model.

Therefore, it will be seen that among the numerical approaches used in solving dam failure, a comparative study is not conducted on turbulence models, as an important and effective variable on modeling the flow caused by dam failure on the erodible bed. Thus, this study emphasized on solving RANS equations, and compared the efficiency of single-equation turbulence kinetic energy models and two- equations kinetic energy-loss ($k - \varepsilon$) in simulation of positive wave motion caused by the dam failure. Flow-3D software version 10.0.1 was used for simulation. The results obtained from the model is compared with the results of the experimental model of Spinewine (2005).

Governing Equations

This study was based on numerical computational modeling using Flow-3D version 10.0.1. The results obtained from the model was verified with results of Spinewine's experimental model (2005). Then, the efficiency of the two turbulent flow models of $k - \varepsilon$ and RNG were compared as effective factors on the accuracy of the results of numerical model. The governing relationships on the flow turbulence models are as follows. It should be noted that all relationships of turbulent flow are used in Flow-3D software in combination with FAVOR's area and volume relations.

Single-equation model of turbulent kinetic energy

This model is a single-equation model for the turbulence scales and it is based on the calculation of turbulence kinetic energy K in terms of normal Reynolds stresses.

To calculate K (in non-isotropic case), the transfer equation is used as follows:

$$\frac{\partial K}{\partial t} + C_1 u \frac{\partial K}{\partial x} + C_2 v \frac{\partial K}{\partial y} + C_3 w \frac{\partial K}{\partial z} = P_t - \frac{CK^{1.5}}{l_m} \quad (1)$$

$$C_1 = \frac{A_x}{\nabla_f}, C_2 = \frac{A_y}{\nabla_f}, C_3 = \frac{A_z}{\nabla_f} \quad (2)$$

$$P_t = P_{1t} + P_{2t} + \text{diff}_t \quad (3)$$

$$\varepsilon = \frac{CK^{1.5}}{l_m} \quad (4)$$

In the above relations, A , ∇_f , P_t and diff_t are respectively FAVOR's area and volume function, turbulence generation and diffusion.

The kinetic energy –turbulence loss model (k-ε):

Two-equations turbulence models are models that use independent transfer equations for length scales of turbulence or turbulence parameters (such as kinetic energy and loss). These models are based on two fundamental assumptions. In a (k-ε) model it is assumed that turbulence viscosity is proportional with the square of turbulent energy divided on turbulence loss (Celik, 1999):

$$\mu_t \propto \frac{\rho k^2}{\varepsilon} \quad (5)$$

Turbulence production- loss equations in a stable form are as follows (Mohammadi and Pironneu,1994) :

$$\frac{\partial k}{\partial t} + \bar{U} \cdot \nabla - \nabla \cdot (D_k \nabla k) + \gamma_k k = F_k \quad (6)$$

$$\frac{\partial \varepsilon}{\partial t} + \bar{U} \cdot \nabla_\varepsilon - \nabla \cdot (D_\varepsilon \nabla_\varepsilon) + \gamma_\varepsilon \varepsilon = F_\varepsilon \quad (7)$$

$$D_k = \frac{\nabla_T}{\sigma_k} + \nabla_L, D_\varepsilon = \frac{\nabla_T}{\sigma_\varepsilon} + \nabla_L \quad (8)$$

$$(9) \quad \gamma_k = \frac{\varepsilon}{k}, \gamma_\varepsilon = \frac{\varepsilon}{k}$$

$$F_k = \frac{\nu_t}{2} |\nabla \bar{U} + \nabla \bar{U}^T|^2, F_\varepsilon = \frac{C_1 k}{2} |\nabla \bar{U} + \nabla \bar{U}^T|^2 \quad (10)$$

In 6-10 relationships, U^r is the mean Reynolds, ν_t is the turbulence viscosity, K is the turbulence kinetic energy, D_k and D_ε are respectively the production diffusion coefficient and turbulence loss coefficient, γ_k and γ_ε are respectively production reflection coefficient and turbulence loss reflection coefficient, F_k and F_ε are the spring's terms. Also, $C_1=0.126$, $C_2=1.92$, $\sigma_k=1$, $\sigma_\varepsilon=1.3$. It should be noted that diffusion and reflection coefficients and spring's terms have non-negative values for physical solutions.

$$\frac{x}{t\sqrt{gH_0}} = X, \frac{x}{H_0} = X^* \quad (11)$$

$$\frac{H}{H_0} = H^* \quad (12)$$

$$\frac{t}{\sqrt{\frac{H}{g}}} = t^* \quad (13)$$

$$\frac{X^*}{t^{*2}} = a^*, \frac{X^*}{t^*} = U^* \quad (14)$$

In relationships (11) to (14) x , t , H_0 , H , X , H^* , t^* , U^* , a^* are respectively horizontal distance, the time spent from the beginning of the experiment, the initial depth of water under the valve (in upstream) H is the height of free surface at t second, non-dimensional variables of horizontal distance, non-dimensional variable of height of free surface, non-dimensional variable of the progressing velocity of wave front and non-dimensional variable of the progressing acceleration of positive wave front.

Findings

After doing simulations by using turbulence kinetic energy models, and k-ε in Flow-3D software, the results are provided in diagrams, it should be noted that relations (11) to (14) are used in diagrams preparation.

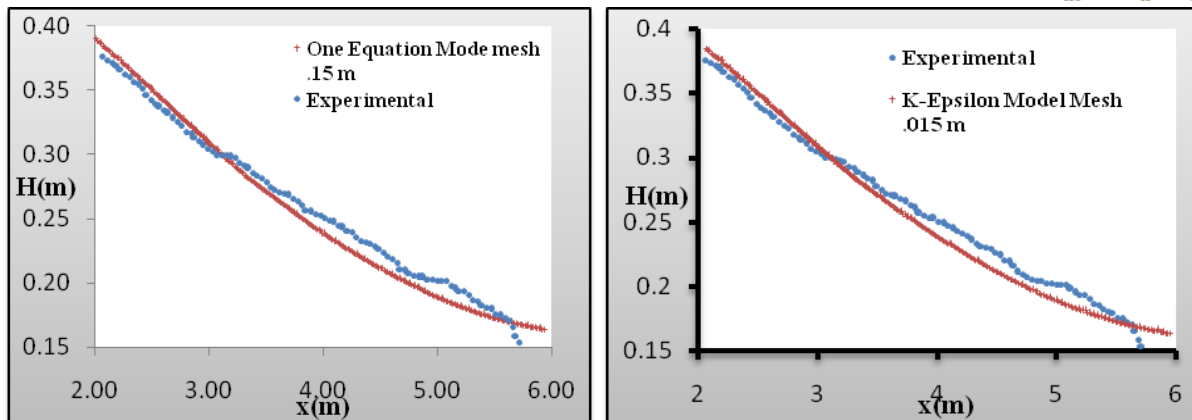


Figure (2)- Profiles of water free surface in turbulence kinetic energy models (rightside) and K-Epsilon(leftside) and the experimental results (t=1.25 s)

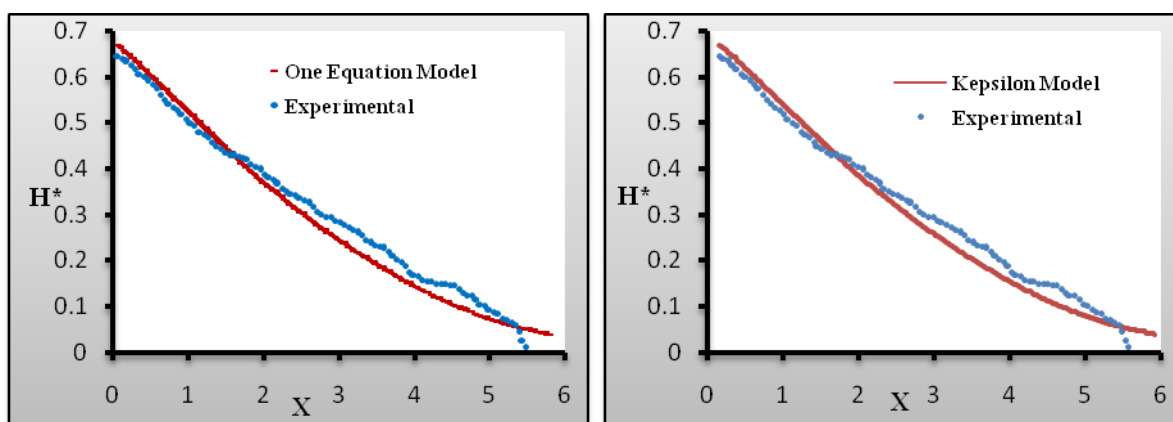


Figure (3)- Non-dimensional profiles of water free surface in turbulence kinetic energy models (rightside) and K-Epsilon(leftside) and the experimental results (t=1.25 s)

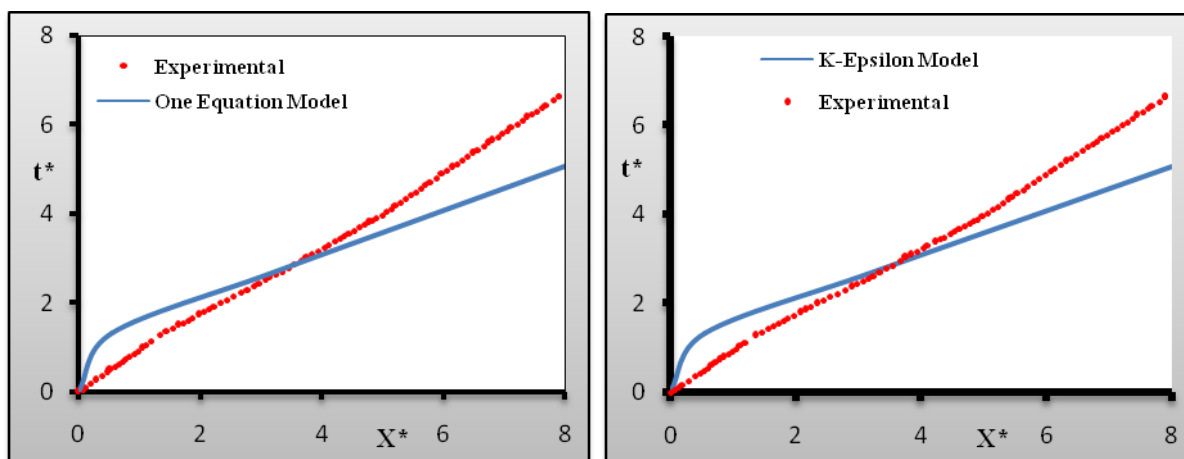


Figure (4)- Non-dimensional profiles of distance- progressing time of positive wave front in turbulence kinetic energy models (rightside) and K-Epsilon(leftside) and the experimental results

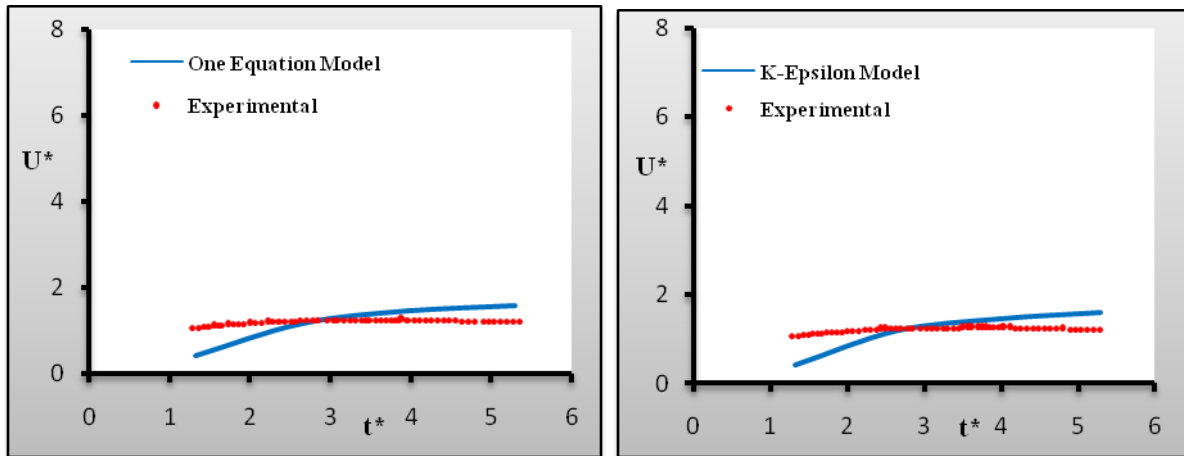


Figure (5)- Non-dimensional profiles of velocity- progressing time of positive wave front in turbulence kinetic energy models (rightside) and K -Epsilon(leftside) and the experimental results

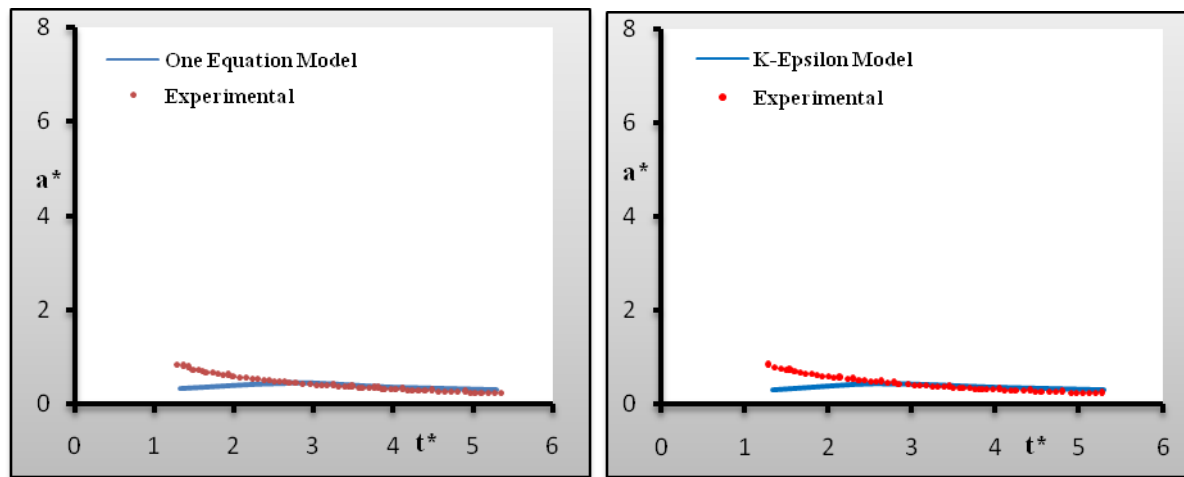


Figure (6)- Non-dimensional profiles of acceleration- progressing time of positive wave front in turbulence kinetic energy models (rightside) and K -Epsilon(leftside) and the experimental results

The accuracy of models in simulations and predictions about the profiles of water free surface is measured based on R^2 criterion and the normal relative error percentage:

$$\varepsilon_L = \sqrt{\frac{\sum_{i=1}^n (\phi_{exp_i} - \phi_{mod_i})^2}{\sum_{i=1}^n (\phi_{exp_i}^2)}} \quad (15)$$

In equation (13), ε_L is the percentage of normal relative error and ϕ_{exp_i} is the obtained value from the experimental results and ϕ_{mod_i} is the obtained variable from the modeling results.

Table (1)- The value of calculated normal error and R^2 for diagrams (2) to (6) in turbulence kinetic energy models and K -Epsilon

Model Name	Accuracy criteria	Hight of free surface	U^*	a^*
Turbulence kinetic energy	R^2	0.886	---	---
	ε_L	0.036	0.326	0.782
K -Epsilon	R^2	0.906	---	---
	ε_L	0.034	0.326	0.782

References

- Ancey, C., Iverson, R.M., Rentschler, M. and Denlinger, R.P. (2008). An Exact Solution for Ideal Dam-Break Floods on Steep Slopes. *Journal of Water Resources Research*, Vol.44, W01430, PP.1-10.
- Celik, I.B. (1999). *Introductory Turbulence Modeling*. Lecture Notes, West Virginia University, Mechanical and Aerospace Engineering Dept, 99P.
- Chow, V.T. (1959). *Open-Channel Hydraulics*. McGRAW-HILL Book Company, INC. 680P.
- Cozzolino, L., Cimorelli, L., Covelli, C., Della Morte, R. and Pianese, D. (2015). The Analytic Solution of the Shallow-Water Equations with Partially Open Sluice Gates: the Dam-Break problem. *Journal of Advances in Water Resources*, Vol.80, PP.90-102.
- Di Cristo, C., Leopardi, A. and Greco, M. (2010). Modeling Dam Break Granular Flow. *Proc. of International Conference of River Flow*, Vol.1, PP.577-583.
- Dressler, R.F. (1952). Hydraulic Resistance Effect Upon the Dam-Break Functions. *Journal of Research of the National Bureau of Standards*, Vol. 49, No. 3, PP. 217-225.
- Gladstone, C., Ritchie, L.J., Sparks, R.S. and Woods, W. (2004). An Experimental Investigation of Density-Stratified Inertial Gravity Currents. *Journal of Sedimentology*, Vol.51, PP.767-789.
- Karthik, T.S.D. (2011). *Turbulence Models and Their Applications*. Indian Institute of Technology., MADRAS, 52P.
- Kocaman, S. and Ozmen-Cagatay, H. (2015). Investigation of dam-break induced shock waves impact on a vertical wall. *Journal of Hydrology*, Vol.525, PP.1-12.
- Miller, S. and Chaudhry, M.H. (1989). Dam-Break Flows in Curved Channels. *Journal of Hydraulic Engineering*, ASCE, Vol.115, No.11, PP.1465-1478.
- Mohammadi, B.M. and Pironneu, O. (1994). "Analysis of the k-ε Turbulence Model". *J. Wiley and Sons*. 133P.
- Orszag, S.A. and Yakhot, V. (1986). Renormalization Group Analysis of Turbulence. *Proc. of the International Congress of Mathematicians*, Berkeley, California, PP.1395-1399.
- Ozmen-Cagatay, H. and Kocaman, S. (2011). Dam-Break Flow in the Presence of Obstacle: Experimental and CFD Simulation. *Journal of Engineering Applications of Computational Fluid Mechanics*, Vol.5, No.4, PP.541-552.
- Ritter, A. (1892). Die Fortpflanzung der Wasserwellen. *Z. Ver. deut. Ing.* Vol.36, No.33, PP.947-954.
- Spinewine, B. and Zech, Y. (2003). Dam-Break Waves on a Moveable Bed : A Test Case Exploring Different Bed Materials and an Initial Bed Discontinuity. 3rd IMPACT Workshop (EU Funded Research Project on Investigation of Extreme Flood Processes and Uncertainty), UCL Louvain la Neuve, Belgium.
- Speziale, C.G. and Thangam, S. (1992). Analysis of an RNG Based Turbulence Model for Separated Flows. *NASA Contractor Report 189600*, No.92-3, PP.1-17.
- Spinewine, B. (2005). Two-Layer Flow Behaviour and the Effects of Granular Dilatancy in Dam-Break Induced Sheet Flow. *Ph.D Thesis in Hydraulic Structures*, University of Catholic the Louvain.
- Stoker, J.J. (1957). *Water Waves*. Interscience Publishing, Inc., New York. 609P.
- Townson, J.M. and Al-Salihi, A.H. (1989). Models of Dam-Break Flow in R-T Space. *Journal of Hydraulic Engineering*, ASCE, Vol.115, No.5, PP.561-575.
- Whitham, G.B. (1955). The Effects of Hydraulic Resistance in the Dam-Break Problem. *Proceedings of the Royal Society Lond. A*, PP.399-407.
- Wood, A. and Wang, K-H. (2015). Modeling Dam-Break Flows in Channels with 90 Degree Bend using an Alternating-Direction Implicit Based Curvilinear Hydrodynamic Solver. *Journal of Computer and Fluids*, Vol.144, PP.254-26

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